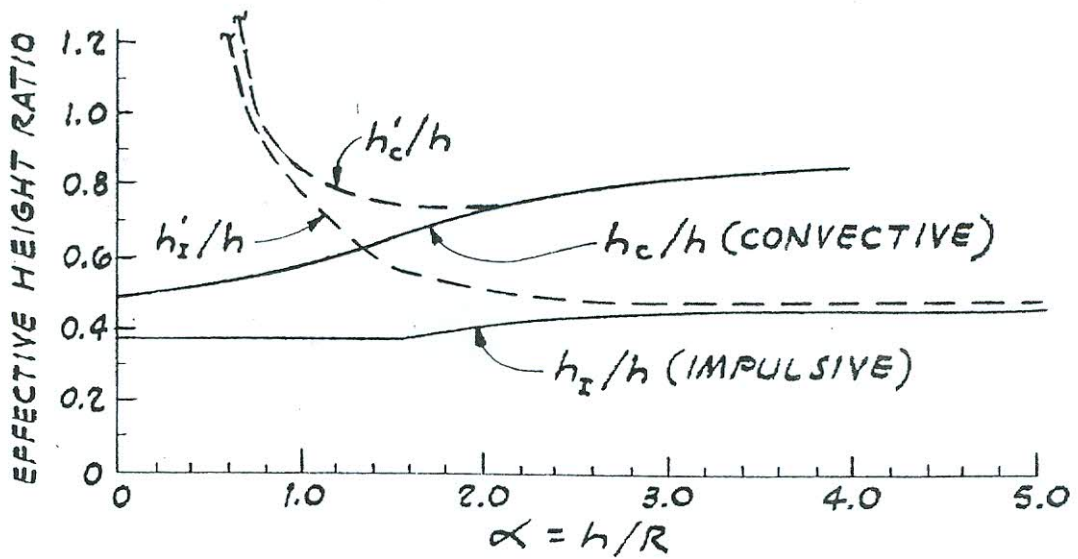


(a). Effective Weight Ratio (See Table 13-1)



(b). Effective Height Ratio (See Table 13-2)

Figure 13-3. Effective weight and height ratios.

| $\alpha = h/R$ | 0.50 | 0.75 | 1.00 | 1.50 | 2.00 | 2.50 | 3.00 | 3.50 | 4.00 | 5.00 |
|-----------------------------|-------------|------|------|------|------|------|------|------|------|------|
| W_i/W , impulsive | 0.29 | 0.42 | 0.54 | 0.71 | 0.79 | 0.83 | 0.86 | 0.88 | 0.89 | 0.91 |
| W_c/W , convective | Cylindrical | 0.66 | 0.53 | 0.43 | 0.30 | 0.23 | 0.18 | 0.15 | 0.13 | 0.11 |
| | Rectangular | 0.69 | 0.58 | 0.48 | 0.34 | 0.26 | 0.21 | 0.18 | 0.15 | 0.13 |
| See Figure 13-3(a) for Plot | | | | | | | | | | |

Table 13-1. Effective weight ratios.

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| $a = h/R$ | | 0.50 | 0.75 | 1.00 | 1.50 | 2.00 | 2.50 | 3.00 | 3.50 | 4.00 | 5.00 |
|-----------------------|-------------|------|------|------|------|------|------|------|------|------|------|
| h_I/h , impulsive | h_I/h | 0.38 | 0.38 | 0.38 | 0.38 | 0.41 | 0.42 | 0.44 | 0.45 | 0.45 | 0.46 |
| h_I/h , impulsive | h_I'/h | 1.60 | 1.00 | 0.80 | 0.58 | 0.51 | 0.49 | 0.48 | 0.48 | 0.47 | 0.47 |
| h_c/h , convective | cylindrical | 0.53 | 0.57 | 0.60 | 0.68 | 0.74 | 0.79 | 0.82 | 0.84 | 0.86 | 0.89 |
| | rectangular | 0.53 | 0.55 | 0.58 | 0.65 | 0.71 | 0.76 | 0.79 | 0.82 | 0.84 | 0.87 |
| h_c'/h , convective | cylindrical | 1.60 | 0.96 | 0.79 | 0.73 | 0.75 | 0.79 | 0.82 | 0.84 | 0.86 | 0.89 |
| | rectangular | 2.00 | 1.11 | 0.86 | 0.73 | 0.74 | 0.77 | 0.80 | 0.82 | 0.84 | 0.87 |

See Figure 13-3(b) for Plot

Table 13-2. Effective height ratio.

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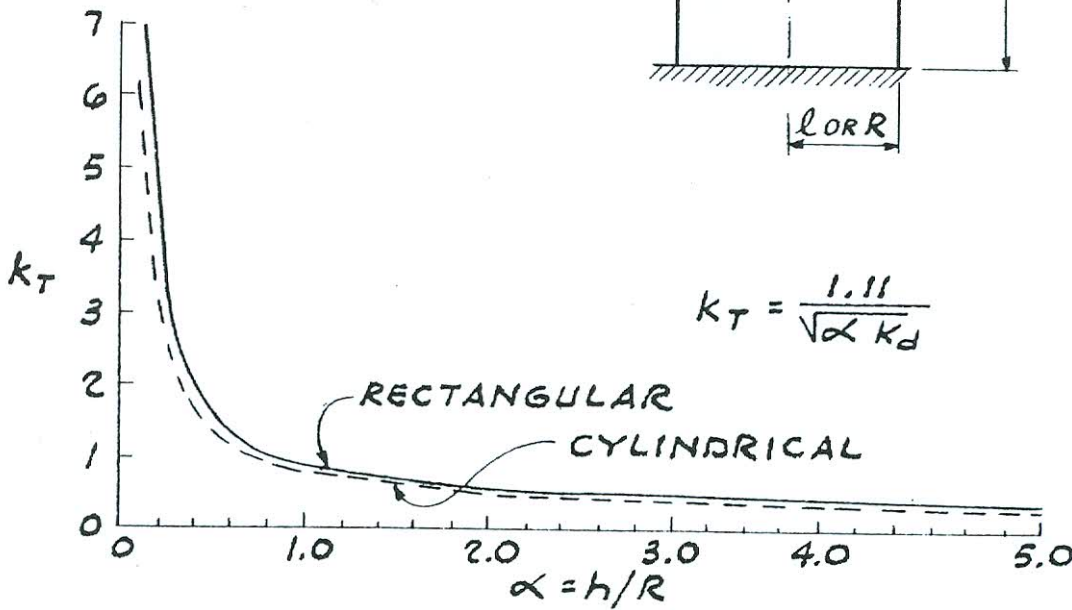
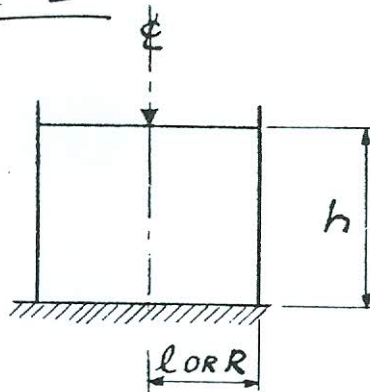


Figure 13-4. Period constant, k_T .

This is consistent with modal analysis procedures where spectral responses of the predominant modes are combined in such a manner.

(4) Sloshing wave height d_{max} . The value of d_{max} must be less than the freeboard height (h_r minus h) for the simplified hydrodynamic procedure to be valid. If d_{max} is greater than (h_r minus h), liquid will overflow the top of the tank when there is no roof or will be confined by the roof if a

roof exists. When there are interior elements, such as baffles or roof supports, the effects of sloshing liquid on these elements will be considered.

b. Design of tank. The critical items of concern in the seismic design of the tank are the horizontal shear at the base, the overturning and uplift forces at foundations, the compression buckling of the tank shell, and, when tie-downs are used, the resulting additional stresses at the attachment of

| α | k_T^* | | | | | | | | |
|-----------------------------|---------|------|------|------|------|------|------|------|------|
| | 0.50 | 0.75 | 1.00 | 1.50 | 2.00 | 2.50 | 3.00 | 4.00 | 5.00 |
| $k_{T, \text{cylindrical}}$ | 1.40 | 1.00 | 0.84 | 0.67 | 0.58 | 0.52 | 0.47 | 0.41 | 0.37 |
| $k_{T, \text{rectangular}}$ | 1.50 | 1.10 | 0.92 | 0.73 | 0.63 | 0.56 | 0.51 | 0.44 | 0.39 |

*used for sloshing (convective motion) period: $T = k_T \sqrt{h}$, where h is the height in feet.
See Figure 13-4 for Plot

Table 13-3. Period constant, k_T

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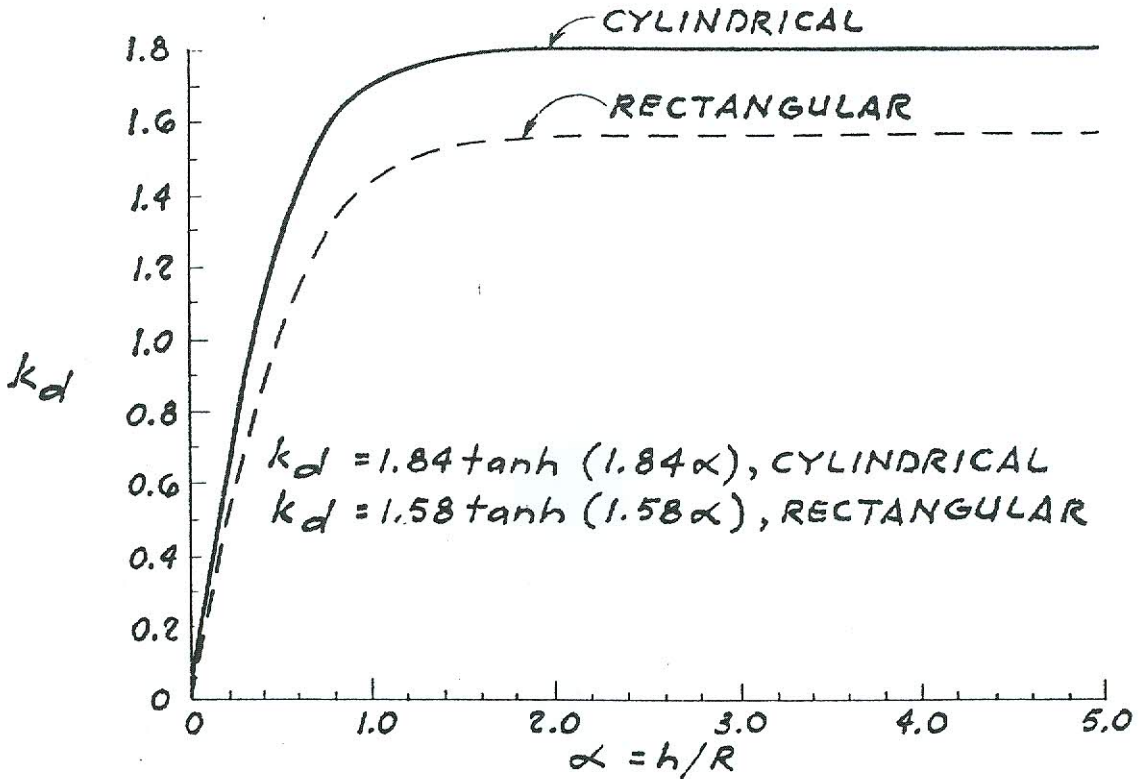


Figure 13-5. Coefficient k_d

| α | 0.50 | 0.75 | 1.00 | 1.50 | 2.00 | 2.50 | 3.00 | 4.00 | 5.00 |
|-------------------|------|------|------|------|------|------|------|------|------|
| k_d cylindrical | 1.33 | 1.62 | 1.75 | 1.83 | 1.84 | 1.84 | 1.84 | 1.84 | 1.84 |
| k_d rectangular | 1.04 | 1.31 | 1.45 | 1.55 | 1.57 | 1.58 | 1.58 | 1.58 | 1.58 |

See Figure 13-5 for Plot

Table 13-4. Coefficient k_d

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